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**PRAIRIE VIEW A. AND M. COLLEGE**

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**SCHOOL OF ENGINEERING**  
**RESEARCH REPORT**

THE EFFECT OF YIELD STRENGTH  
AND DUCTILITY TO FATIGUE DAMAGE

by  
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CASE FILE  
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## ABSTRACT

The cumulative damage of aluminium alloys with different yield strength and various ductility due to seismic loads has been studies. The responses of an idealized beam with a centered mass at one end and fixed at the other end to El Centro's and Taft's earthquakes are computed by assuming that the alloys are perfectly elastoplastic materials and by using numerical technique. Consequently, the corresponding residual plastic strain can be obtained from the stress-strain relationship. The revised Palmgren-Miner cumulative damage theorem is utilized to calculate the fatigue damage. The numerical results show that in certain cases, the high ductility materials are more resistant to seismic loads than the high yield strength materials. The results also show that if a structure collapse during the earthquake, the collapse always occurs in the very early stage.

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## INTRODUCTION

There are two different approaches to structural design. One is based on the concept of allowable stress and elastic behavior of materials and the other on the concept of ultimate load and inelastic behavior. Since the structure designed by using plastic theory is more economic than those by applying elastic theory, plastic design concept has been gradually accepted by most engineers and has been widely applied in constructions.

If the external forces are large enough, the plastic-design oriented structures, undoubtedly, will undergo plastic deformation and create residual plastic strain. The residual plastic strain will gradually accumulate if the structure is subjected to dynamic loads such as wind force, seismic loads, etc. If the cumulative plastic strain reaches the critical values, the structures will collapse. This process is usually termed low cycle fatigue failure. Hence the study of low cycle fatigue failure is of importance to the structures designed by using the concept of plastic theory.

There are many papers dealing with inelastic behavior of materials (2, 9, 10, 12, 13)\* and the fatigue damage of structures under the action of seismic loads. (4, 5, 6, 10). Few of them have stressed the importance of the comparison of different fatigue life among the same materials with different yield strength and ductility.

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\*The numerals in parenthesis refer to the list of references.

Since low cycle fatigue damage can be expressed in terms of plastic strain, the fatigue life of structural systems can always be improved by choosing the materials with high ductility. However, high ductile materials generally have low yield strength which will cause the larger deformation than the materials with high yield strength. It becomes a complicated problem to choose material for structural members to resist dynamic loading. Based on the given random behavior of plastic deformation, a method for evaluation of alternative materials to be used in structures subjected to random White Noise excitation was presented (15). Since there is not any white noise excitation in reality, it is desirable to study the fatigue damage of materials subjected to earthquake load in more details.

The purpose of this investigation is to study the fatigue damage of materials with different yield strength and various ductility due to seismic loads. The revised Palmgren-Miner cumulative damage theorem expressed in terms of plastic strain is used to compute the fatigue damage. As an example, aluminium alloys (5052 and 3030 groups) are considered on an ideal beam, subjected to 1948 El Centro's and 1951 Taft's earthquakes.

## LOW CYCLE FATIGUE DAMAGE MODEL

In computing the fatigue damage, the Palmgren-Miner criterion (8), because of its simplicity, has been most widely used during the past decades. This theorem can be mathematically expressed as follows:

$$\sum_{i=1}^N d_i = D \quad <1>$$

Where  $d_i$  is the fatigue damage during its cycle, and  $D$  is the total cumulative damage under  $N$  cycles. The theorem states that the fatigue failure will occur if the total damage  $D$  reaches some critical level, which depends on material properties.

At present, this study is concentrated on the low cycle fatigue life which usually is referred to the case with life less than  $10^5$  cycles. If a structure is supposed to collapse within such a limited cycle, the magnitude of cyclic loading may be large enough to cause the plastic strain in the materials. Hence the Palmgren-Miner criterion for low cycle fatigue life was developed to associate with plastic strain. Manson and Gross et. al. (3, 7) proposed the following low cycle fatigue life criterion by introducing plastic strain for predicting the total number of cycles of reversed-strain to cause fatigue failure:

$$N^m (\Delta e_t) = C \quad <2>$$

Where  $\Delta e_t$  is the plastic tensile strain,  $m$  is a constant depending on material properties.  $N$  is the fatigue life and  $C$  is

some constant. Later Yao and Munse (14) suggested that, for uniaxially loaded metal, the cumulative damage can be expressed in terms of material ductility and plastic strains as follows:

$$\sum_{i=1}^N \left\langle \frac{(\Delta e_t)}{e_f} \right\rangle^{1/m} = 1 \quad <3>$$

Where  $e_f$  is the ultimate plastic strain and is constant for a given material,  $\Delta e_t$  is the plastic strain at tensile cycle and  $m$  is material constant dependent on ambient and loading rate. Usually  $m$  is defined as function of plastic strain in tensile and compressive cycles, denoted by  $e_c$  and  $e_t$ , respectively.

$$1/m = 1 - 0.086 \left( \frac{\Delta e_t}{\Delta e_c} \right) \quad <4>$$

The cyclic history of plastic strains in tensile and compressive cycles is illustrated in Figure 1. Equation <3> and <4> will be considered as the fatigue damage criterion through this investigation.

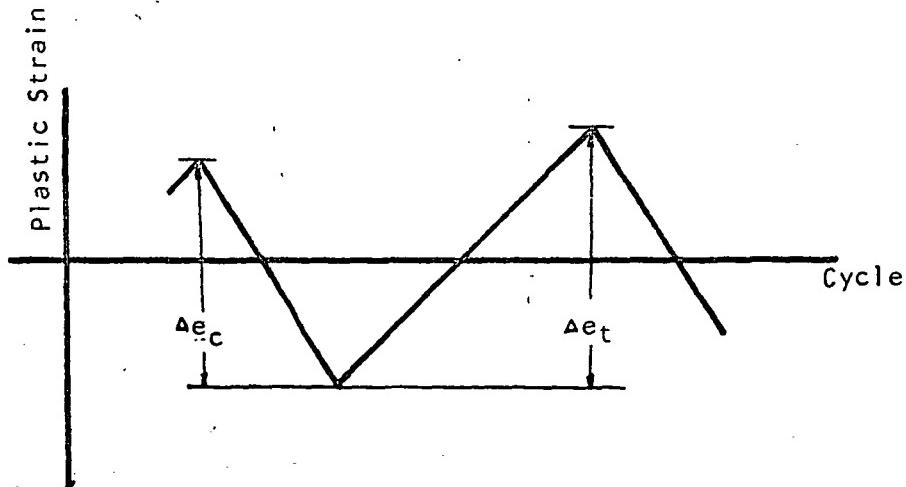


Figure 1. Cyclic History of Plastic Strain

## MECHANICAL MODEL

Consider a beam with a centered mass at one end and fixed at the other end subjected to a fluctuating excitation as shown in Figure 2.

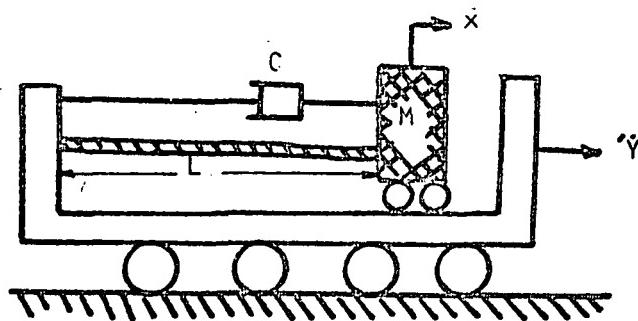


Figure 2. Mechanical Model

The equation of motion of this spring-dash-pot system is well known.

$$M \ddot{x} + C \dot{x} + G(x) = M \ddot{y} \quad <5>$$

Where  $M$  is the concentrated mass of the beam.

$C$  is the damping of the mechanical system.

$x$  is the deformation of the beam.

$\ddot{y}$  is the accelerogram of seismic loads.

$G(x)$  is the forced-deformation function dependent on the stress-strain curve.

The force-deformation function,  $G(x)$ , can be derived from the stress-strain relationship of a given material. For simplifying the computation, the aluminium alloys considered here are assumed to be elasto-plastic materials. The stress-strain diagram of such a material is shown in Figure 3.

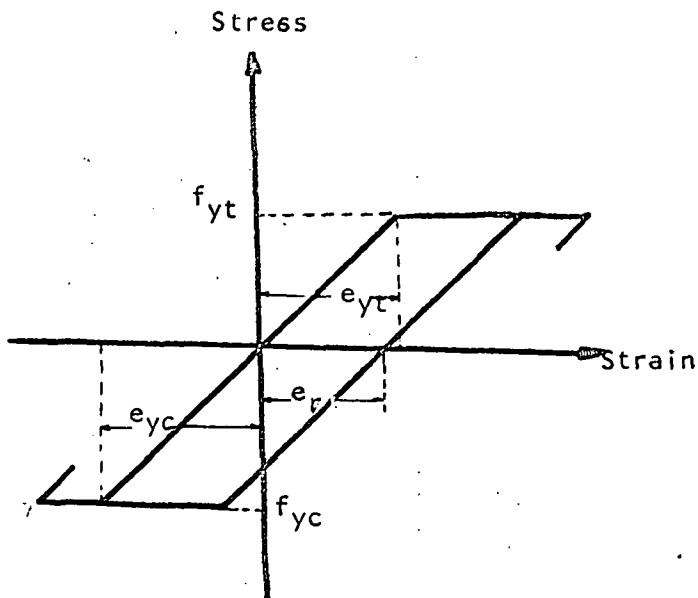


Figure 3. Elasto-plastic stress-strain diagram

Let  $A$  be the cross-sectional area of the beam.

$E$  be the modulus of elasticity.

$L$  be the length of beam.

$e_r$  be the residual strain.

$e_y$  be the yielding strain.

$f_{yt}$  be the yielding tensile stress.

$f_{yc}$  be the compressive yielding stress.

If  $f_{yt} = f_{yc} = f_y$ , the force-deformation function can be formulated as follows:

$$G(x) = EA \left( \frac{x}{L} - e_r \right) \quad \text{If } \left| \frac{x}{L} - e_r \right| < |e_y|$$

$$G(x) = A f_y \quad \text{If } \left| \frac{x}{L} - e_r \right| > |e_y| \quad <6>$$

Since the analytical solution of equation (5) is not available, Ruge-Kutta Numerical intergretion was used to obtain the deformation,  $x$ . From the known deformation and stress-strain curve, the cyclic history of plastic strain can be found, then the fatigue damage can be calculated from equation <3> .

## NUMERICAL EXAMPLES

In order to illustrate the low cycle fatigue damage of structures subjected to dynamic loads, several aluminium alloys are studied. The mechanical properties of these materials are tabulated in Table (1).

Alloy	Yield strength ksi	Ductility %	Modulus of Elasticity $10^3$ ksi
Alclad 3003 - H12	18	20	10.0
Alclad 3003 - H14	21	16	10.0
Alclad 3003 - H16	25	14	10.0
Alclad 3003 - H18	27	10	10.0
5052 - H32	28	18	10.0
5052 - H34	31	14	10.0
5052 - H36	35	10	10.0
5052 - H38	37	8	10.0

Table 1. Mechanical Properties of Aluminium Alloys

Since the natural frequency can not be well defined if the structural systems allow to undergo plastic deformation, the terminology of "artificial frequency" is adopted. Let  $p_a$  be

the artificial frequency which is defined as  $\sqrt{(AE) / (ML)}$ . If both sides of equation <5> are divided by the mass M, it becomes

$$\ddot{x} + 2 c p_a \dot{x} + g(x) = \ddot{y} \quad <7>$$

Where  $c$  is the damping ratio,  $p_a$  is the artificial frequency and  $g(x)$  is  $G(x)/M$ . It is to be noted that  $g(x)$  can be reformulated in terms of artificial frequency.

In order to make the comparision, two seismic loads are utilized in this study. One is 1948 El Centro's earthquake NS component with maximum peak in the order of 0.312g, the other is 1951 Taft's earthquake S69E component with a peak value in the order of 0.157g, where g denotes the gravitational force. Because of the high peak value of El Centro's earthquake, the artificial frequency of the mechanical system varies from 50 rad/sec to 70 rad/sec, and for the case of Taft's earthquake, the frequency varies from 35 rad/sec to 45 rad/sec. With the length of beam, L, being 10 inches and the information shown in Table 1, the fatigue damages of aluminium alloys subjected to El Centro's and Taft's earthquakes are computed by the aid of 360/65 IBM computer. The computer program is listed in Appendix. The results are tabulated in Tables 2, 3, and 4, and also plotted in Figures 4, 5, and 6.

Cumula-	Alloy	5052 - H32	5052 - H34	5052 - H36	5052 - H38
Fre-	quency				
50	2.9920149	3.1137094	2.5031681	3.9078865	
52.5	1.5593367	1.5319510	1.7257175	1.8590279	
55	0.97939056	1.1105976	1.4277601	1.2899484	
57.5	0.85163534	0.96612877	1.1536770	0.82214361	
60	0.62915808	0.66661555	0.44669420	0.58149391	
62.5	0.32020706	0.32498884	0.4687804	0.31088388	
65	0.32253897	0.32985848	0.14389163	0.1470437	
67.5	0.25728476	0.17950523	0.13773221	0.13794059	
70	0.12210464	0.090874553	0.06572252	0.06312008	

Table 2 Fatigue Damages of 5052 Group Alloys Subjected to

E1 Centro's Earthquake

Cumula- tive Alloy damage Frequency	Alclad 3003 - H12	Alclad 3003 - H14	Alclad 3003 - H16	Alclad 3003 - H18
52.5	3.9831467	4.1246414	4.5256672	4.8264341
55	2.9735933	3.6988564	1.9604654	2.4367723
57.5	2.9497690	2.8602915	1.5088053	2.1295109
60	2.5558205	1.7559586	1.2991552	1.6246128
62.5	1.9535789	1.6068087	1.0175724	0.68900013
65	1.8724232	0.9148501	0.67170912	0.51194036
67.5	1.2662811	0.7284845	0.52865228	0.48792696

Table 3 Fatigue Damages of 3003 Group Alloys Subjected to

E1 Centro's Earthquake

Cumula-	Allloy	5052-H32	5052-H34	5052-H36	5052-H38
tive	damage				
Frequency		18	14	10	8
35	1.9330864	2.2903147	2.4820929	2.7911043	
37.5	1.6505632	1.3604631	1.7209997	1.5659914	
40	1.0455103	0.99359804	0.77765310	0.68719399	
42.5	0.63108575	0.43125600	0.32834977	0.31635483	
45	0.31469911	0.18018502	0.14462858	0.14523304	

Table 4 Fatigue Damages of 5052 Group Alloys Subjected to  
Taft's Earthquake

Figure 4. Fatigue Damages of 5052 group  
alloys due to El Centro's Earthquake

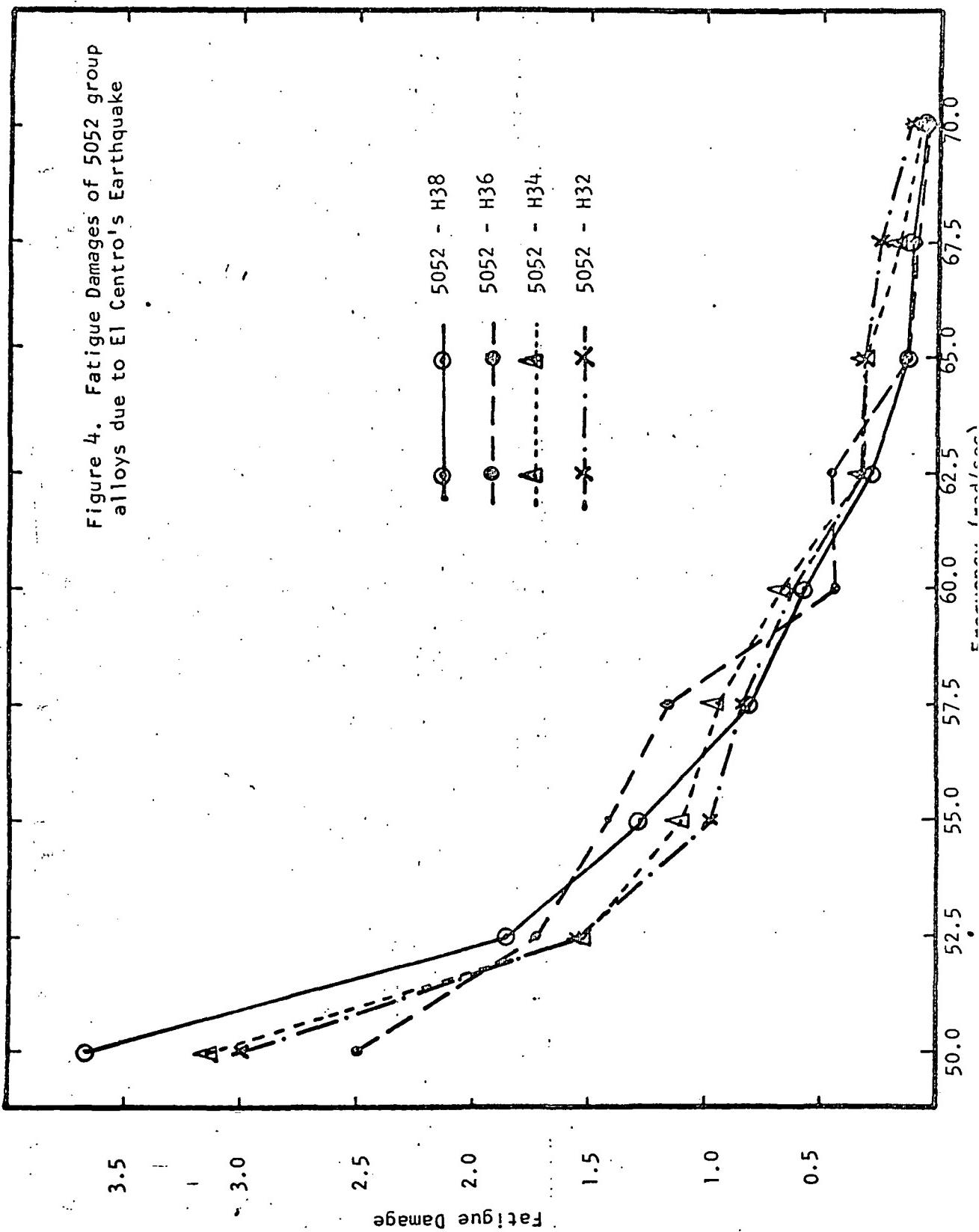
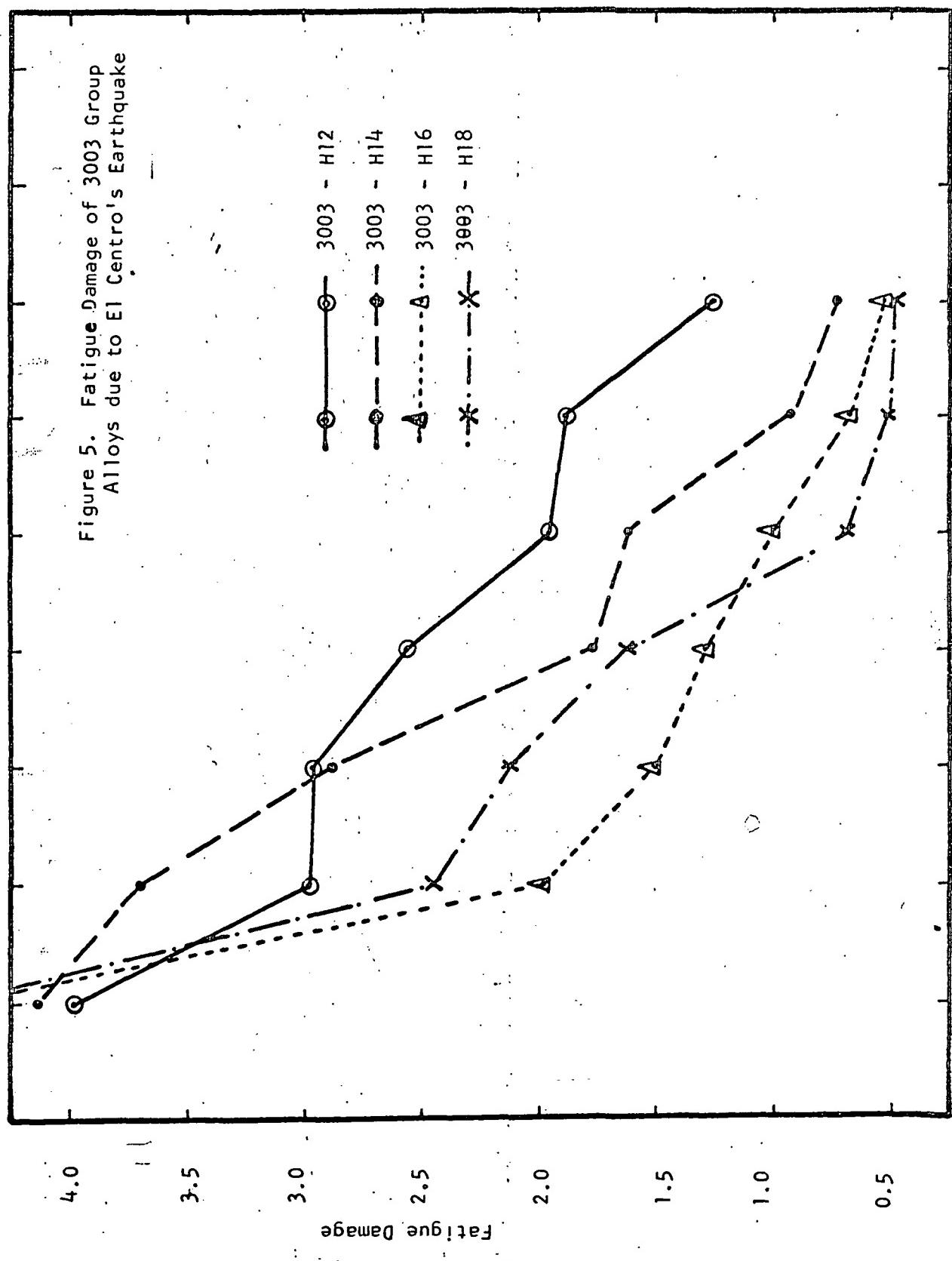


Figure 5. Fatigue Damage of 3003 Group  
Alloys due to El Centro's Earthquake



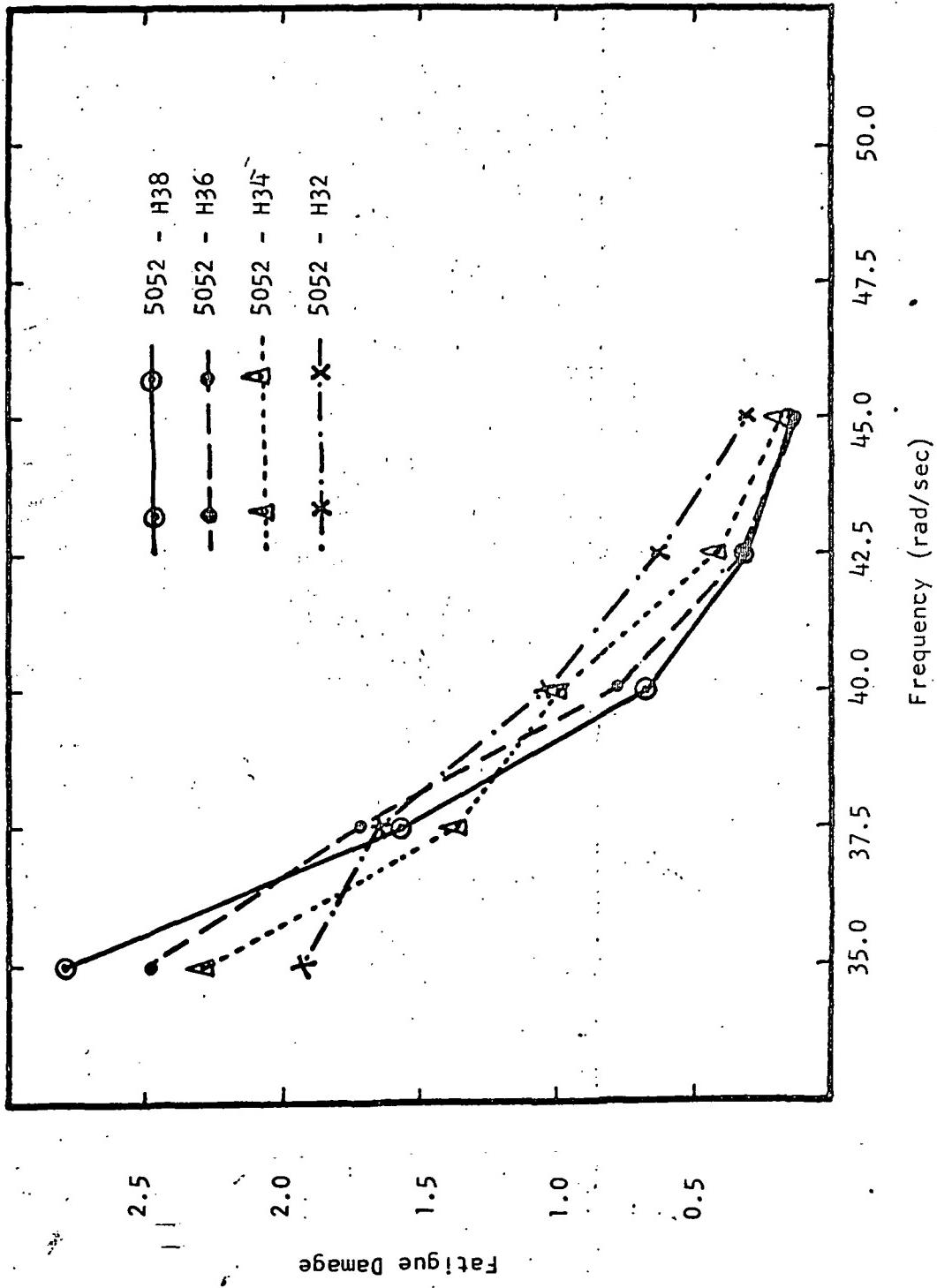


Figure 6. Fatigue Damage of 5052 Group Alloys due to Taff's Earthquake.

## DISCUSSION AND CONCLUSION

For Table 2, 3, and 4 or Figures 4, 5, 6, they show that in general, the fatigue damage will increase as the stiffness of the beam decreases. However, in some cases, the fatigue damages of higher stiffness member are smaller than that of lower stiffness member as it can be seen in frequencies 62.5 rad/sec and 65.0 rad/sec for alloys 5052-H32, and 5052-H36. This phenomenon is due to the fact that the frequency of the beam coincide with the frequency of seismic load.

Since a structural system is usually defined as "collapse" if the cumulative fatigue damage reaches one, the discussion here will concentrate on those materials whose cumulative damage smaller than one. Under Taft's earthquake, the cumulative damages increase as the yield strength of alloys 5052 group decreases regardless the ductility of materials. But it is not the case for the same materials subjected to El Centro's earthquake. The results show that in certain frequency domain, the material with high ductility is stronger to resist seismic load than the material with high yield strength. For easy comparison, the cumulative damage histograms are plotted versus time as shown in Figure 7 and 8 for alloys 5052 group in the frequencies of 60.0 rad/sec and 8 for alloys 5052 group in the frequencies of 60.0 rad/sec and 62.5 rad/sec.

The damage histograms are also plotted for alloys 5052 subjected

to Taft's earthquake in some frequency domain. It is very interesting to note that the cumulative damages pile up very fast in the early stage of earthquake action as shown in Figures 7, 8, 9, and 10. In other words, fatigue damage is always created during the first 5 - second period for the 30-second duration El Centro's earthquake, and the first 12-second period for 60-second duration Taft's earthquake. It can be concluded that if a structure can survive during the first quarter period of earthquake action, it surely will not collapse and under this earthquake. On the contrary, it can be said that if a structure collapses during the earthquake, the failure always occurs in the very early stage.

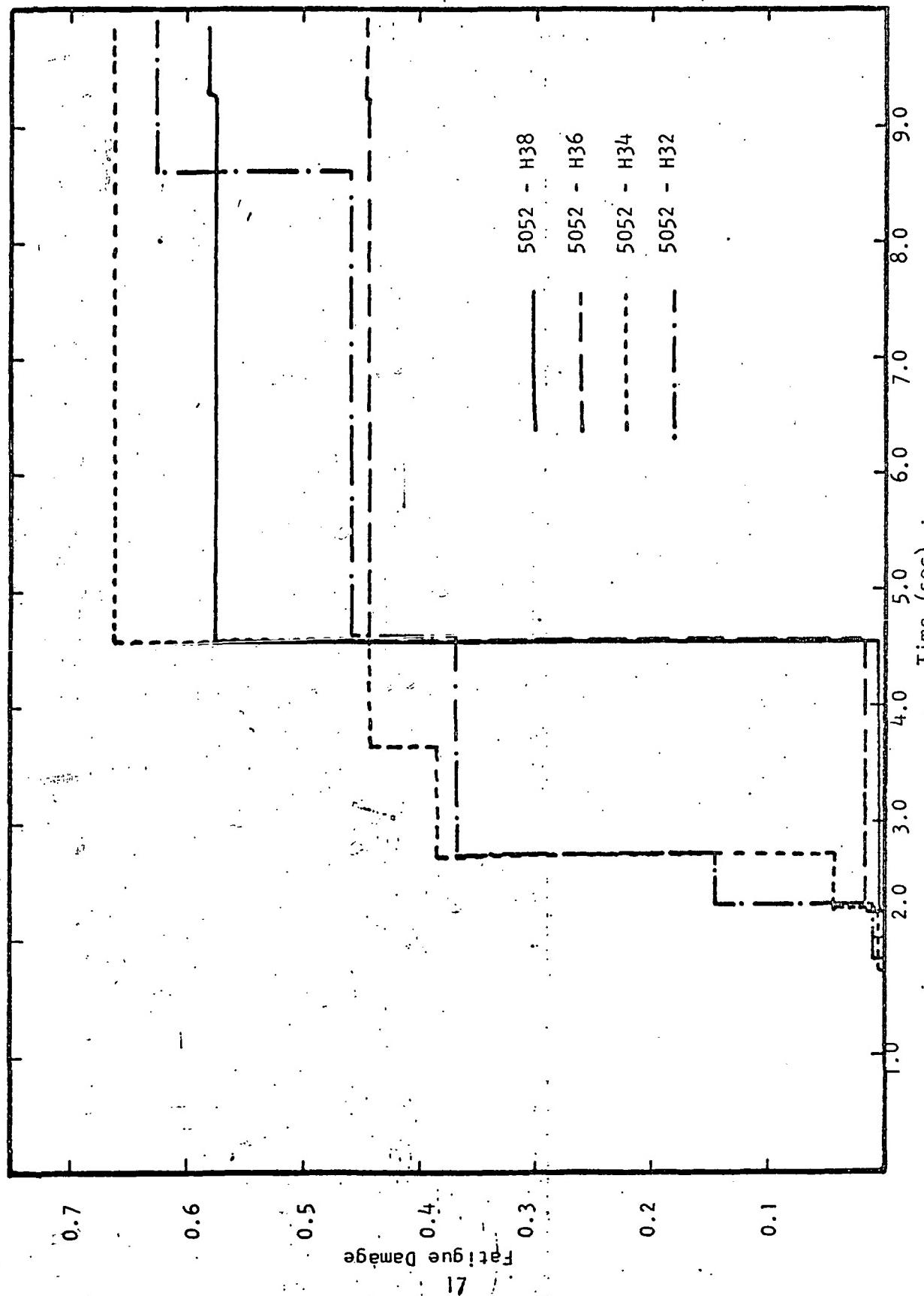


Figure 7. Fatigue Damage Histogram for 5052 Group Alloy's in the Frequency of 60 rad/sec Due to El Centro's Earthquake.

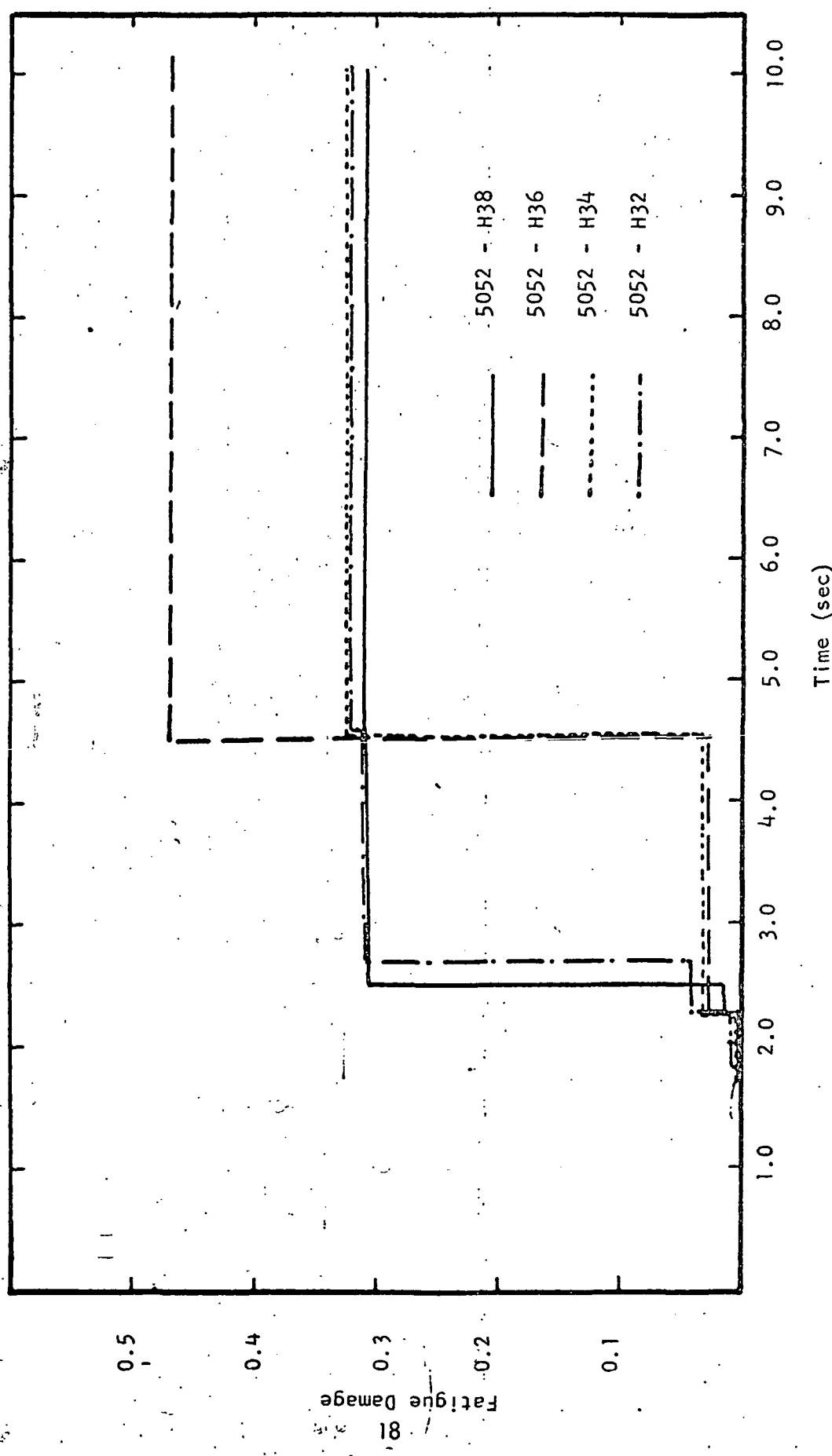


Figure 8. Fatigue Damage Histogram for 5052 Group Alloys in the Frequency of 62.5 rad/sec Due to El Centro's Earthquake.

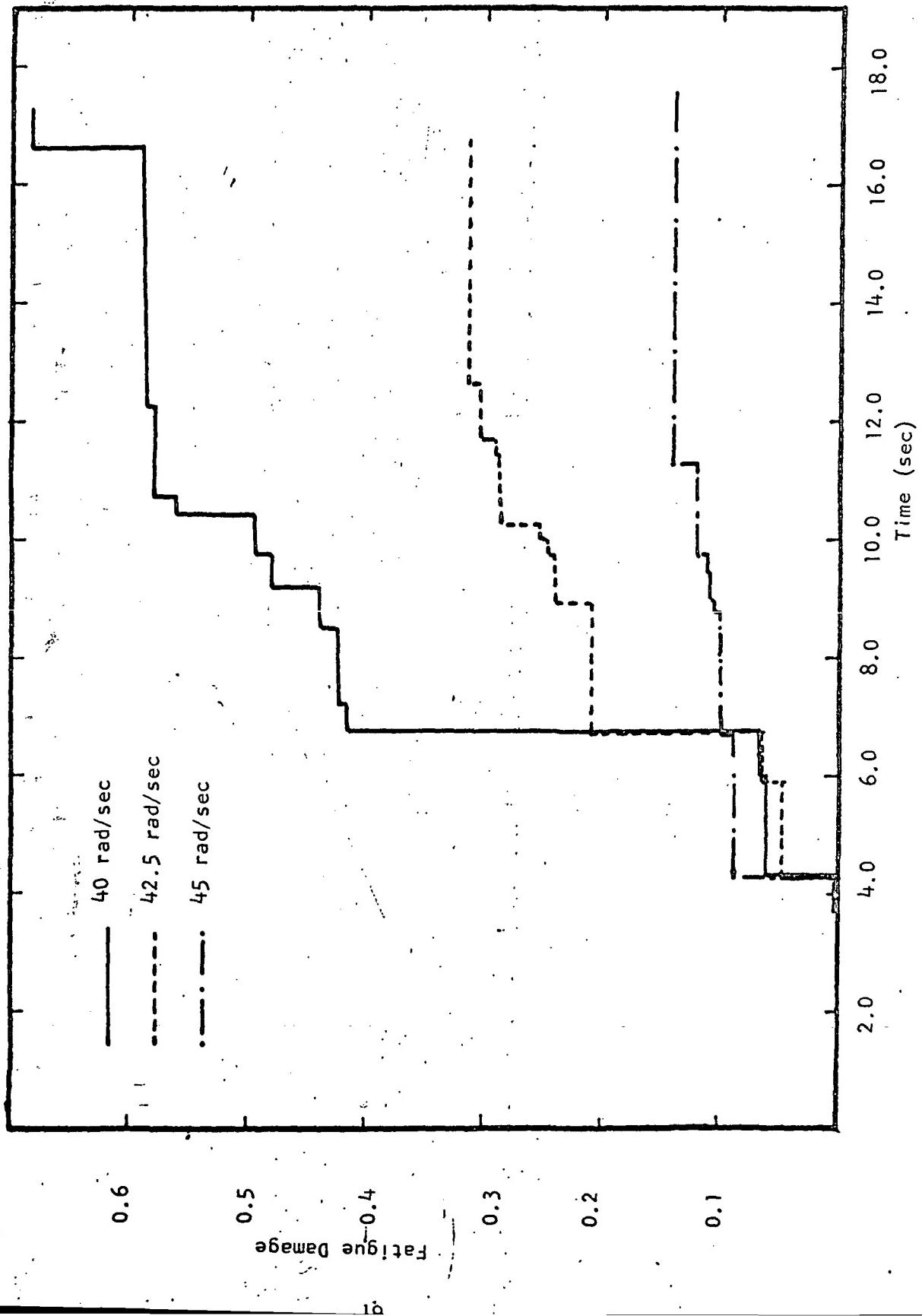


Figure 9. Fatigue Damage History diagram for 5052 - H38 Alloy Due to Taft's Earthquake.

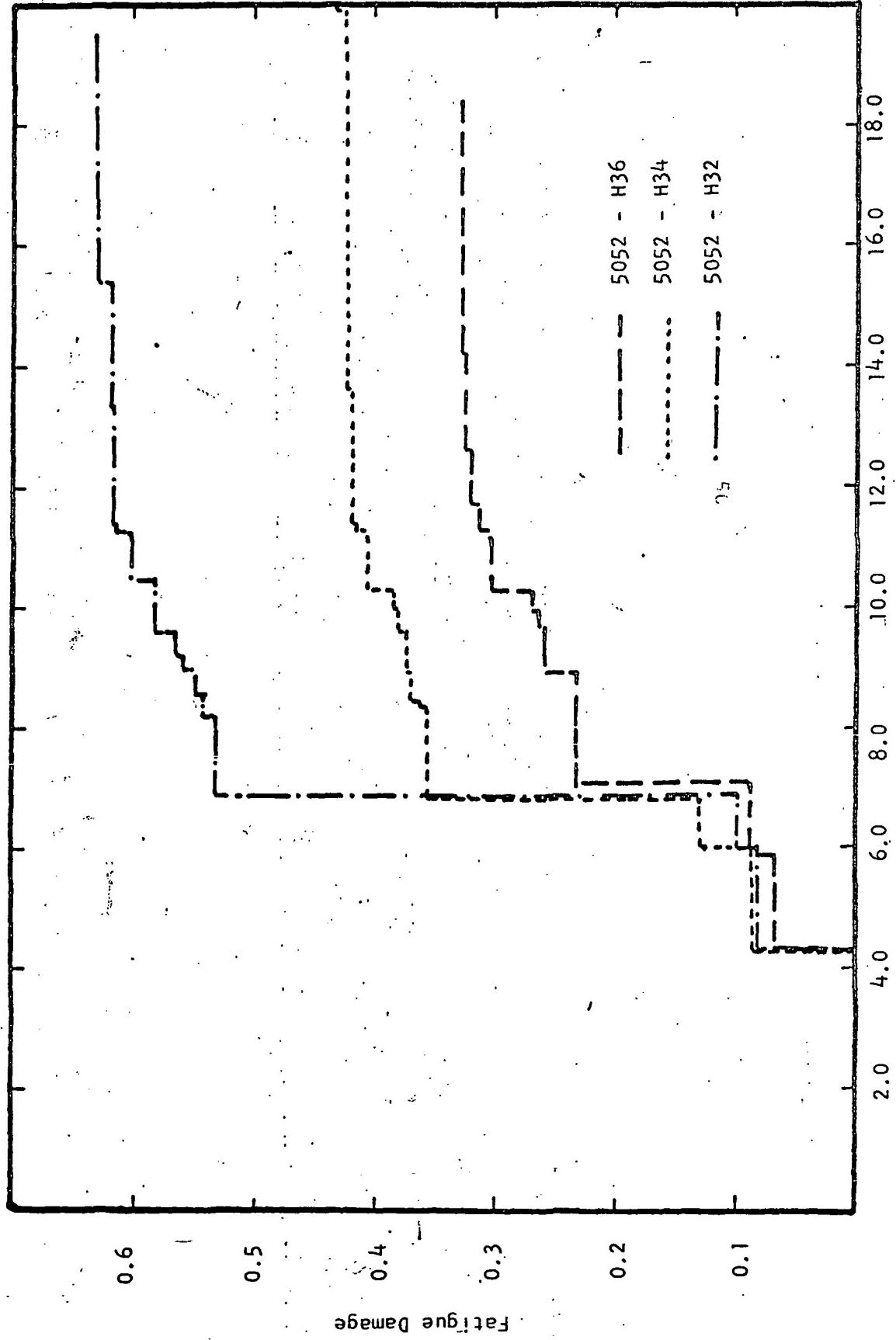


Figure 10. Fatigue Damage Histogram For 5052 Group Alloys in the Frequency of 42.5 rad/sec due to Taft's Earthquake.

ACKNOWLEDGEMENT

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LEVEL 20

MAIN

DATE = 72215

17/18/44

C  
C     FATIGUE DAMAGE DUE TO EARTHQUAKE EXCIATATION (MECHANICAL MODEL)  
C  
C

```

DIMENSION PRMT(5),Y(2),DERY(2),AUX(8,2),YDIS1(3000)
1,S(500),AC(500),ACCEL(3000),XT(3000),YDIS2(3000)
2,OMGA(2),ETA(2),DULTY(2),EYELD(2),CON(2),Q(2),YDMIN(2),YDMAX(2),
3OME(2),SK1(2),SK2(2),QMAX(2),QMIN(2),YMAX(2),YMIN(2),YELD(2),TL(2)
4,DAMGC(2),DAMGV(2),K(2),KT1(3000),KT2(3000)
COMMON/V1/ACCEL,OMGA,ETA
COMMON/VP/YMAX,YMIN,YELD,QMAX,QMIN,Q,SK1,SK2,QME
COMMON/VD/DULTY,TL,NL
COMMON/VO/YDIS1,YDIS2,YDMIN,YDMAX,NT,XT,DMU,TI,APM,BPM,CPM
EXTERNAL FCT,OUTP
READ(5,905) IEN,DEL
905 FORMAT(15,F10.5)
READ(5,906) (PRMT(I),I=1,5)
906 FORMAT(5F10.5)
READ(5,903) (S(I),AC(I),I=1,IEN)
903 FORMAT(4(F6.2,F12.7))
J1=2
IR=0
IFN1=IFN+1
S(IFN1)=PRMT(2)
AC(IFN1)=0.
DO 110 I=1,IEN
DI=(S(I+1)-S(I))/DEL
ID=DI
IID=DI+0.5
IF (ID-IID) 80,90,100
80 ID=ID+1
GO TO 90
100 WRITE(6,904) I
904 FORMAT(2X,12HERROR IN AC ,I3)
ID=ID-1
90 IR=IR+ID
ACCEL(J1-1)=AC(I)*0.3864
DO 120 J=J1,IR
120 ACCEL(J)=ACCEL(J-1)+(AC(I+1)-AC(I))*0.3864/DI
J1=ID+J1
110 CONTINUE
READ(5,907) APM,BPM,CPM
907 FORMAT( 3F10.6)
READ(5,908) N
NDIM=N*2
900 FORMAT(15)
WRITE(6,999)
999 FORMAT(1H1)

```

LEVEL 20

MAIN

DATE = 72215

17/18/44

```

31 READ(5,902)_(TL(I),I=1,N)
902 FORMAT(2F10.5)
READ(5,908) TLMU,LCHECK
908 FORMAT(F10.5,I5)
DO 20 I=1,N
READ(5,901) OMGA(I),ETA(I),DULTY(I),YEYLD(I),CON(I)
901 FORMAT(5F10.5)
YEYLD(I)=YEYLD(I)*TL(I)
Q(I)=0.
YDMIN(I)=0.
YDMAX(I)=0.
QME(I)=OMGA(I)**2*YEYLD(I)
SK1(I)=OMGA(I)**2
SK2(I)=SK1(I)*CON(I)
QMAX(I)=QME(I)
QMIN(I)=-QME(I)
YMAX(I)=YEYLD(I)
YMIN(I)=-YEYLD(I)
WRITE(6,923) OMGA(I),ETA(I),DULTY(I),YEYLD(I),CON(I),TL(I)
923 FORMAT(2X,10HFREQUENCY F10.5,17H DAMPING RATIO F10.5,11H DUCTILI
1TY F10.5,11H YIELDING F10.5,13H STIFFNESS 2 F10.5,3H L F10.5//)
20 CONTINUE
DIM=NDIM
DO 10 I=1,NDIM
Y(I)=0.
DERY(I)=1./DIM
10 CONTINUE
DMU=DEL*TLMU
TT=0.
NT=1
CALL RKGS(PRMT,Y,DERY,NDIM,1HLF,FCT,OUTP,AUX)
NL=NT
CALL DAMAGE(YDIS1,YEYLD(1),DAMGC(1),DAMGV(1),K(1),KTIM1,TL(1),
1DULTY(1),NL,KT1,1)
CALL DAMAGE(YDIS2,YEYLD(2),DAMGC(2),DAMGV(2),K(2),KTIM2,TL(2),
1DULTY(2),NL,KT2,2)
TEND1=XT(KTFM1)
TEND2=XT(KTIM2)
WRITE(6,800) TEND1,TEND2
800 FORMAT(2X,3HT1=1PE16.7,10X,3HT2=1PE16.7)
DO 30 I=1,N
WRITE(6,921) N, YDMAX(I),YDMIN(I)
30 WRITE(6,920) N,DAMGC(I),DAMGV(I),K(I)
920 FORMAT(10X,I3,6H STORY,16H DAMAGE FACTOR 1P2E16.7,20H NUMBER OF
1 CYCLES I7//)
921 FORMAT(10X,I3,6H STORY,19H MAX DISPLACEMENT 1PE16.7,19H MIN DISP
1LACEMENT 1PE16.7//)
K1=K(1)

```

```
K2-K(2)
DO 140 I=1,K1
J=KT1(I)
140 WRITE(6,940) I,XT(J)
DO 150 I=1,K2
J=KT2(I)
150 WRITE(6,950) I,XT(J)
940 FORMAT(2X,12HNO OF CYCLES 15,2X,5HTIME=1PE16.7,2X,2H01)
950 FORMAT(2X,12HNO OF CYCLES 15,2X,5HTIME=1PE16.7,2X,2H02)
IF(LCHECK).31,32,31
32 CONTINUE
STOP
END
```

C SUBROUTINE RKGS(PRMT,Y,DFRY,NDIM,IHLF,FCT,OUTP,AUX)

```
C
C DIMENSION Y(2),DFRY(2),AUX(8,2),A(4),B(4),C(4),PRMT(5)
C DO 1 I=1,NDIM
1 AUX(8,I)=.26666667*DFRY(I)
Y=PRMT(1)
XEND=PRMT(2)
H=PRMT(3)
PMT(5)=0.
CALL FCT(X,Y,DFRY)
```

C ERROR TEST

```
1 IF(H*(XEND-X))38,27,2.
```

C PREPARATIONS FOR RUNGE-KUTTA METHOD

```
2 A(1)=.5
A(2)=.2928932
A(3)=1.707107
A(4)=.1666667
B(1)=2.
B(2)=1.
B(3)=1.
B(4)=2.
C(1)=.5
C(2)=.2928932
C(3)=1.707107
C(4)=.5
```

C PREPARATIONS OF FIRST RUNGE-KUTTA STEP

```
DO 3 I=1,NDIM
```

```
AUX(1,I)=Y(I)
```

```
AUX(2,I)=DFRY(I)
```

```
AUX(3,I)=0.
```

```
3 AUX(6,I)=0.
```

```
TREC=0
```

```
H=H+H
```

```
IHLF=-1
```

```
ISTEP=0
```

```
IEND=0
```

C START OF A RUNGE-KUTTA STEP

```
4 IF((X+H-XEND)*H)7,6,5
```

```
5 H=XEND-X
```

```
6 IEND=1
```

LEVEL 20

RKGS

DATE = 72080

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C RECORDING OF INITIAL VALUES OF THIS STEP

7 CALL OUTPI(X,Y,DERY,19FC,NODIM,PRMT)

1F(PRMT(5))40,8,40

9 ITEST=0

9 ISTEP=ISTEP+1

C

C

C START OF INNERMOST RUNGE-KUTTA LOOP

J=1

10 AJ=A(J)

RJ=R(J)

CJ=C(J)

DO 11 I=1,NODIM

R1=H\*DERY(I)

R2=AJ\*(R1-RJ\*AUX(6,I))

Y(I)=Y(I)+R2

R2=R2+R2+R2

11 AUX(6,I)=AUX(6,I)+R2-CJ\*R1

1F(J-4)12,15,15

12 J=J+1

1F(J-3)13,14,13

13 X=X+.5\*H

14 CALL FCT(X,Y,DERY)

GOTO 10

C END OF INNERMOST RUNGE-KUTTA LOOP

C

C

C TEST OF ACCURACY

15 1F(ITEST)16,16,20

C

C

C IN CASE ITEST=0 THERE IS NO POSSIBILITY FOR TESTING OF ACCURACY

16 DO 17 I=1,NODIM

17 AUX(4,I)=Y(I)

ITEST=1

ISTEP=ISTEP+ISTEP-2

18 THLF=HLF+1

X=X-H

H=.5\*H

DO 19 I=1,NODIM

Y(I)=AUX(1,I)

DERY(I)=AUX(2,I)

19 AUX(6,I)=AUX(3,I)

GOTO 9

C

C

C IN CASE ITEST=1 TESTING OF ACCURACY IS POSSIBLE

20 IMOD=ISTEP/2

1F(ISTEP-IMOD-IMOD)21,23,21

21 CALL FCT(X,Y,DERY)

G LEVEL - 20

RKGS

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DO 22 I=1,NDIM  
AUX(5,I)=Y(I)  
22 AUX(7,I)=DERY(I)  
GOTO 9

C COMPUTATION OF TEST VALUE DELT

23 DELT=0.

DO 24 I=1,NDIM  
24 DELT=DELT+AUX(8,I)\*ABS(AUX(4,I)-Y(I))  
IF(DELT-PRMT(4))28,28,25

C ERROR IS TOO GREAT

25 IF(IHLF=1)126,36,36

26 DO 27 I=1,NDIM

27 AUX(14,I)=AUX(5,I)

ISTEP=ISTEP+ISTEP-4

X=X-H

IFEND=0

GOTO 18

C RESULT VALUES ARE GOOD

28 CALL FCT(X,Y,DERY)

DO 29 I=1,NDIM

AUX(1,I)=Y(I)

AUX(2,I)=DERY(I)

AUX(3,I)=AUX(6,I)

Y(I)=AUX(5,I)

29 DERY(I)=AUX(7,I)

CALL OUTP(X-H,Y,DERY,IHLF,NDIM,PRMT)

IF(PRMT(5))40,30,40

30 DO 31 I=1,NDIM

Y(I)=AUX(1,I)

31 DERY(I)=AUX(2,I)

IFFC=IHLF

IF(IFEND)32,32,39

C INCREMENT GETS DOUBLED

32 IHLF=IHLF-1

ISTEP=ISTEP/2

H=H+H

IF(IHLF)4,33,33

33 IMOD=ISTEP/2

IF(ISTEP-IMOD-IMOD)4,34,4

34 IF(DELT-.02\*PRMT(4))35,35,4

35 IHLF=IHLF-1

ISTEP=ISTEP/2

H=H+H

GOTO 4

C. RETURNS TO CALLING PROGRAM.

36 IHLF=11

CALL FCT(X,Y,DERY)

GOTO 39

37 IHLF=12

GOTO 39

38 IHLF=13

39 CALL OUTP(X,Y,DERY,IHLF,NDIM,PRMT)

40 RETURN

FND

SUBROUTINE FCT(X,Y,DERY)

DIMENSION Y(4), DERY(4), ACCEL(3000),

1OMGA(2),ETA(2),YMAX(2),YMIN(2),YELD(2),QMAX(2),QMIN(2),Q(2),SK1(2)

2,SK2(2),QME(2)

COMMON/VP/YMAX,YMIN,YELD,QMAX,QMIN,Q,SK1,SK2,QME

COMMON/VI/ACCEL,DMGA,ETA

XN=X/0.01+1.

NX=XN

TEMP=NX

CALL PLAS(QF1,Y(1),YMAX(1),YMIN(1),YELD(1),QMAX(1),QMIN(1),Q(1),  
1SK1(1),SK2(1),QME(1))

CALL PLAS(QF2,Y(2),YMAX(2),YMIN(2),YELD(2),QMAX(2),QMIN(2),Q(2),  
1SK1(2),SK2(2),QME(2))

DERY(1)=Y(2)

DERY(2)=-2.\*ETA(1)\*OMGA(1)\*Y(2)-QF1-ACCEL(NX)-(ACCEL(NX+1)-  
1ACCEL(NX))\* (XN-TEMP)+2.\*ETA(2)\*OMGA(2)\*Y(4)+QF2

DERY(3)=Y(4)

DERY(4)=-4.\*ETA(2)\*OMGA(2)\*Y(4)-2.\*QF2+2.\*ETA(1)\*OMGA(1)\*Y(2)+QF1

RETURN

END

LEVEL 20

MAIN

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C

SUBROUTINE DAMAGE(Y,YELD,DAMGC,DAMGV,K,KTIME,TL,DULTY,NL,KT,IDX)

DIMENSION Y(3000),DPP(3000),DPN(3000),KT(3000)

KTIME=0

DO 401 I=1,NL

DPP(I)=0.

—401— DPN(I)=0.

SK=1./TL

YMAX=0.

YMIN=0.

EPP=0.

EPN=0.

DAMGV=0.

DAMGC=0.

N=1

J=1

402 IF (ABS(Y(N))-YELD) 403,403,404

403 N=N+1

—402— IF (N-NL) 402,402,420

404 IF (Y(N)) 419,405,405

405 YYE=Y(N)-YELD

406 EPP=YYE\*SK

YMAX=Y(N)

DPP(J)=EPP-EPN

KT(J)=N

407 N=N+1

IF (N-NL) 408,408,420

408 IF (Y(N)-YMAX) 410,409,409

409 YYE=YYE+Y(N)-YMAX

GO TO 406

410 YYN=Y(N)-(YMAX-2.\*YELD)

IF (YYN) 411,407,407

411 EPN=YYN\*SK

YMIN=Y(N)

IF (DPP(J)) 412,414,413

412 WRITE(6,999)

999 FORMAT (10X,16H ERROR IN DPP(J))

RETURN

413 J=J+1

414 DPN(J)=EPN-EPP

415 N=N+1

IF (N-NL) 416,416,420

416 IF (Y(N)-YMIN) 417,417,418

417 YYN=YYN+Y(N)-YMIN

EPN=YYN\*SK

YMIN=Y(N)

GO TO 414

418 YYE=Y(N)-YMIN-2.\*YELD

EVEL 20.

DAMAGE

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IF (YYE) 415,415,406  
419 YYN=Y(N)+YELD  
GO TO 411  
420 K=J  
DO 422 J=1,K  
IF (DPP(J)) 421,422,421  
421 CGES=1.-0.86\*DPN(J)/DPP(J)  
DAMGV=DAMGV+(DPP(J)/DULTY)\*\*CGES  
IF (KTIME.NE.0) GO TO 423  
IF (DAMGV.GE.1.) KTIME=KT(J)  
423 DAMGC=DAMGC+(DPP(J)/DULTY)\*\*1.86  
WRITE(6,998) DAMGC,DAMGV,J,IDX  
998 FORMAT(2X,6HDAMGC=1PE16.7,2X,6HDAMGV=1PE16.7,2X,12HNO OF CYCLES  
115,15,5HSTORY)  
422 CONTINUE  
RETURN  
END

SUBROUTINE PLAS(QM,Y,YMAX,YMIN,YELD,QMAX,QMIN,Q,SK1,SK2,QME)  
IF (Q) 212,201,208  
201 IF(ABS(Y)-YELD) 202,202,203  
202 QM=SK1\*Y  
GO TO 216  
203 IF(Y) 204,206,206  
204 QM=QMIN+(Y-YMIN)\*SK2  
205 Q=-1.  
YMIN=Y  
QMIN=QM  
GO TO 216  
206 QM=QMAX+(Y-YMAX)\*SK2  
207 Q=1.  
YMAX=Y  
QMAX=QM  
GO TO 216  
208 IF (Y-YMAX) 209,206,206  
209 YM2=YMAX-Y-2.\*YELD  
IF (YM2) 210,211,211  
210 QM=QMAX+(Y-YMAX)\*SK1  
GO TO 216  
211 QM=QMAX-2.\*QME-YM2\*SK2  
GO TO 205  
212 IF (Y-YMIN) 204,204,213  
213 YM2=Y-YMIN-2.\*YELD  
IF (YM2) 214,214,215  
214 QM=QMIN+(Y-YMIN)\*SK1  
GO TO 216  
215 QM=QMIN+2.\*QME+YM2\*SK2  
GO TO 207  
216 RETURN  
END

SUBROUTINE OUTP(X,Y,DERY,IHLF,NDIM,PRMT)

DIMENSION Y(4),DERY(4),PRMT(5),YDIS1(3000),YDIS2(3000),XT(3000)  
1,YDMAX(2),YDMIN(2)  
CCMMON/VN/YDIS1,YDIS2,YDMIN,YDMAX,NT,XT,DMU,T-T,APM,BPM,CPM  
N=NDIM/2  
DO 212 I=1,N  
J=2\*I-1  
IF (Y(J)) 211,212,213  
211 IF(YDMIN(I)-Y(J)) 212,212,214  
214 YDMIN(I)=Y(J)  
GO TO 212  
213 IF (YDMAX(I)-Y(J)) 215,212,212  
215 YDMAX(I)=Y(J)  
212 CONTINUE  
IF (IHLF-10) 216,216,217  
217 WRITE(6,910).X,IHLF  
910 FORMAT (10X,2HX=,F10.5,10X,5HIHLF=,13)  
RETURN  
216 IF(X-TT) 227,228,228  
228 XT(NT)=X  
YDIS1(NT)=Y(1)  
YDIS2(NT)=Y(3)  
NT=NT+1  
TT=TT+DMU  
227 IF (NT-3000) 225,226,226  
225 CHEK=ABS(Y(1))  
DO 219 I=2,NDIM  
IF (CHEK-ABS(Y(I))) 219,219,218  
218 CHEK=ABS(Y(I))  
219 CONTINUE  
220 IF(CHEK.EQ.0.) RETURN  
IF (CHEK-1.0E-03) 221,221,222  
221 PRMT(4)=APM  
RETURN  
222 ICHEK=CHEK  
IF (ICHEK) 223,224,223  
224 CHEK=CHEK\*10.  
GO TO 222  
223 CHEKI=ICHEK  
PRMT(4)=CHEK/CHEKI\*BPM  
RETURN  
226 WRITE (6,911).X  
911 FORMAT (10X,BHNT=3000 F10.5)  
PRMT(5)=1.  
RETURN